# GIB LEMKURT DEMODULATOR 

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#### Abstract

The Federal Communications Commission of the United States requires that the geographical coordinates of the antenna site be furnished on applications for a radio station license and construction permit. Microwave transmission engineers regularly determine the coordinates of a site by carefully plotting the site on a topographic map and measuring its location relative to known lines of latitude and longitude. Two methods of measurement are in common use. Site coordinates can be determined by either method to the degree of accuracy expected by the Commission.


Whenever someone describes the Iocation of something, he may mention how far and in what general direction it is from some other identiliable and lamiliar object, structure or landmark. Thus, a larmer might deseribe the location of his water well as so many pares due talst of a certain gatr in his fromer line. With this description anyone knowing where the lence gate is and which direction is east cond probalbly find the well without diflienlty. But a stranger to the area would likely consider this hind of information insullizcims.

Another method that is commonly used to retate where something is localod, is to deline it in lerms of its territorial boundaries. Most inhabited places in the world ran be quite precisely located by means of ant address. An address is nothing more than an orderly listing of the various welldefined houndaries within which a place is situated. The postal services of the world rely on the publie's use of such a lormat to designate the place of origin and the destination of the maik. As long as the address is correct and complete, delivery of the mail to the proper location is gencrally assured.

There is yet another and more precise mrans of pimpointing places on the globe that navigators, surveyors, construction enginerrs, carlographers, and others interested in the many features of the earth's surlace are inclined to use: the grographical coordinater system of latitude and longitude. Within this system the location of any place, whether it be on land or water, can be identified with as great a degree of accuracy as the aceuracy of the tools with which the measurements of distance and direction are made. The Federal Commmencations Commission ( FCC ) ol the United States reguires that this geegraphical coordinate system bee used in the application for a radios station construction permit to record where the station's transmitting antema or antrona tower is to be placed. The coordinates of the site are to be aceurate to the nearest second or within onte second of latitude and longitude, depending on the type of ractio service. "How do 1 go about detormining the geographical coordinates ol a site to that degree of accuracy?" is a question that probably arises in the minds ol most applicants filing for a radio station license for the


Figure 1. Lines of lalitude (parallels) appear on the surface of a global map of the carth as concentric circles parallel to the equator. Lines of longitude (meridians) appear as greal circles that pass through the north and south poles.
lirst time. One answer to the question is oflired in the following paragraphs.

## The Coordinate System

The present-day greographical coordinate system has its roots in a work of the seeond century Eigyptian historian and grographer, P'tolemy. In his athas he catalogued places and positionpel them on a map of his contemporary world by moans of latitude and longitude. Similarly, on maps of today's world, the horizontal position ol a place is fixed by its relation to recognized standard prarallels of latitude and standard meridians of longitude. Parallels and meridians are simply imaginary lines. certain ones of which are commonly traced out on llat and groblal maps of the world.

Figure 1 shows how these lines appear on a global mapr. This typr of map presents a spherical view of the earth - the earth, in fact, is not a perlert sphere, but an oblate spheroid - on which parallels of latitude appear as toncentric circles drawn paralled to the equator, the reference paralled, aboul the grobe's axis ol rotation. Heridiane of longitude appear on the globe ds circles, generally refiered to as great circles, converging on and passing
through the north and south poles. The most widely acepted referencer or prime meridian is that half of a great circle extending from the two poles on which lies a hill, the former site of the Royal (irernwich Observatory in (ireenwich, England. But in Russia the prime meridian at times has been takern to be the meridian that passes through the Kremlin. In this discussion the Grenwich meridian and the equator are taken to ber respectively the roference meridian and the reference parallel. Since they serve as the relerente lines in the grographical coordinate system, the (ireenwieh meridian is designated the zero degree $\left(0^{\circ}\right)$ line of longitude, and the equator is designated the zeros degree of latitude. All other lines of longitude and latitude are reckoned in mits of angular are (degrees, minutes, seconds) from these two reference lines. All other lines of longitude lie so many units ol angular art: fast or west of the Greenwich meridian. Likewise, all other lines of latitude: lie so many units of angular are north or south of the equator. The meridian of $180^{\circ}$ lies halfway around the earth from the Greenwich meridian and, except near certain land masses, serves as the international date
line: The two poles, north and south points on the earth's axis of rotation, are the north and south parallets of $90^{\circ}$. Thus, in the geographical coordinate system, every spot on the earth's surface can be said to be somewhere from $0^{\circ}$ to $90^{\circ}$ north or south of the rquator and from $0^{\circ}$ to $180^{\circ}$ east or west of the (ireenwich meridian. Of course, no north or south direction is griven any point located on the equator: meither is an cast or west direction given a point on the Greenwich meridian. But the coordinates of all other points on the face of the globe have a north, south, cast, or west direction from these two refirener lines.

One aspect of the grographical coordinate system that reportedly puzales many is the use of degrees, minutes. and seconds in designating the coordinates of a point. I'art of the bewilderment appears to stem from a misunderstanding of the numerical relationship between the three units of measure. There may also le a difficulty in grasping why these units of measure are considered appropriate to this coordinate systrm.

Students lcarn in plane geometry that the degree is a unit used to measure angles. lts value is based on a division of the circumferenee of a circle into 360 ares of equal lengel. The eentral angle formed ly two radii subtended by one of these ares is equal to one degree ( $1^{\circ}$ ). If a one degree are is divided into 60 ares of "plual length, then the central angle subtended by one of these ares is equal to one minute ( $1^{\prime}$ ). If a one minute are is divided into 60 ares of equal length, then the erntral angle subtended by one of these ares is equal to one second (1"). Central angles subtended by ares less than one second in length are ordinarily measured in decimal fractions of a second. Figure 2 depicts these relationships. In summary, this
system of angular measurement is based on the division of the central angle of a circle into 360 degrees, a degree into 60 minutes, a minute into 60 sreonds, and a second into decintal fractions. It is to be noted that these numerical dimensions of an angle are purely arbitrary, that the division of the circle inte 360 equal parts probaby originated with ancient astronomers who observed that the earth completed its circle of orbit around the sun in approximately 360 days. However, in some areas of the world today, the circle is divided into 400 rather than 360 degrees of arc.

Since the units of angular measurement are lased on an arbitrary division of the circumberence of the circle, a plane geometric ligure, their use in a coordinate system devised to locate points on the surface of a sphere, the related solid grometric figure, should not appear inappropriate. Inasmuch as the warth can be viewed as a spheregranted that it is more aceurately described as an oblate spheroid or - Hlipsoid flattened at the poles because of the centrifigal force caused by the rarth's rotation around its axis the degrer, minute and second as units of angular measurement can properly be applied to the geographical coordinato systrm.

## Maps

The majority of individuals interested in obtaining the geographic coordinates of a site are not equipped with the instruments, hnowledge, and experience required to derive site coordinates from measurements made in the field. Conseguently, they will ordinarily rely entirely on the work dome and the information compiled by one or more agencies of the local, state, provincial, or national govermments. In most parts of the world these agencies conduct numerous types of surveys to


Figure 2. The degree, minute, and second are units of angular measurement whose dimensions are based on arbitrary dirisions of the circumference of the circle. The circumference is commonly divided into 360 one degree arcs (A). Eath one degree of are is further divided into 60 one minute ares (B). Each one minute of are is then divided in 60 one second ares (C).
gather pertinent geodetic, grological, and geographic data and to develop from such data all sorts and sizes of maps and tables. The agencies then make copies of most of these maps and tables available for purchase by the public.

The Congress of the United States has established an agency in the l)epartment of the Interior, the United States Ceological Survey, to develop and publish maps of appropriate scale and accuracy for all of the country. This agency has bren engaged for many decades in the production of several series of general-purpose topographic maps from survey information collected by its own survey teams as well as by those of other local, state and Federal agencies, notally the United States Coast and (ieodetic Survey. These maps are printed on rectangular sheets of paper that vary in size according to the series of the map and the part of the country presented. The two series of quadrangle maps mosi
suited fur determining site coordinates are the $7.1 / 2$-minute and 15 -minute series. The first of these series presents torrestrial areas bounded by seven-and-a-half mirintes of tatitude and longitude; the second, areas bounded by fiftern minutes. Maps in the 7-1/2 minute series are drawn to a scale or map-to-earth-surface ratio of 1 : 24,000 ; neps in the 15 -minute series to a scale of I: 62,500 . Topographic maps of more than three quarters of the United States are now available in one or the other of these series. The cutire csountry will eventually be covered by maps in either of these series. Indexes of published maps are available free on request to the U.S. Geological Survey, Washington, D.C. 20242, or IIS. Geological Survey, Federal Center, Denver, Colorado 80225.

All maps in these series are based on the North American horizontal datum of 1927 and the National Geodetic vertical datum of 1929. A datum
in surveying torminology is a measured reference point to which all other measurements in a survey are made. The North American horizontal datum is the Meades Ranch triangulation station located about 12 miles north of lucas, Kansas. The [.S. Coast and Ceodetic Survey in 1927 rstablished this station as the permanent reforence point for its transcontinental triangulation survey. It is now the datum for all triangulation networks of the entire North American contiment. By means of astronomical and geodetic observations made with precision instruments, and from subsequent computations, the latitude and longitude of this station and its azimuth to another nearby permanent station, Wadlo, were determined to one thousandth of a second accuracy. The Vational Grodetic vertical datum is mean sea level, which is established as the average height of the surface of the sea for all stages of the tide measured over a 19-year period.

In addition to these horizontal and vertical datums, U.S. Geological Survey topographie maps use the Clarke spheroid or ellipsoid of 1866 as the reference for the size and shape of the earth's surface at average sta level Alexander Clarhe, a Scottish geodesist, computed the length of the polar axis of this representative figure of the earth to be $41,710,242$ feet and the equatorial axis to be $41,852,124$ feet in length. Since then other values that are now considered more accurate have bern computed.

Although these topographic maps are based on quite aceurately measured horizontal, vertical, and surface references, the plotting of information, based on these references, onto maps must not be construed to lor done to the same degree of accuracy. The national standards adopted for horizontal accuracy require that at
drast 90 precent ol the welledefined map points shall be plotted correctly within one-fiftieth of an inch on the published maps. The standards for vertical accuracy require that at least 90 percent of the elevations interpolated from the contour lines shall be eorrect within one-hall the contour interval. Contour intervals vary with the terrain being mapped.

Since the FCC requires that microwave towers and antenna sites be plotted on L'S. Geological Survey mapsif atailable, and accompany the request for a station construction permit and license, it has becon traditionally assumed that the same maps may also be used to obtain the geographical coordinates of the site. It may be neecesary to conduct a field survey in order to extablish antenna sites that provide a clear transmission path. It may also be necessary to perform a cadastral survey of the sites in order to establish property line boundaries. But oner a site is decided on, its coordinatre ean be determined with sufficient arcuracy from measurements made on a topographic map.

## Methods

Thore are two hasic methods that can be used to determine site coordinates from a topographic map. The accuracy of cither method is dependent primarily on three clements: the accuracy of the map, the aceuracy of the measuring tool, and the accuracy of the measurement itself.

The horizontal accuracy of topographic maps published by the I.S. Geological Survey has already been stated. (Vertical accuracy is not relebant to the determination of site coordinates.) In terms of ground measurement the standard for horizontal accuracy means that at least 90 percent of the well-delined map points on I: 24,000 scale maps are aceurately
plotted within a tolerance of 40 feet on the ground. Points on I: 62,500 scald maps are plotted within a tolerance of 100 feren. One hundred freet is slightly less than one sereond of latitude or longitude measured near the equator and becomes greater than onfe second of latitude as one draws nearer the poles. limmediately, the difficulty of obtaining site coordinates from a map to within one-serond accuracy becomes apparent. Naps of other parts of the world ustally pose a similar, il not greater, difliculty to the precise measurment of coordinates.

The physical condition of a map call also alfect the accuracy of madsurements made on it. Wost purchased maps are printed on paper that can stretch or shrink with freepuent use and exposure to the weather. The dimensions of a map can change if it is mishandled or mistreated.

The tool used to make measurements on the map can also affeet the accuracy with which site coordinates are determined. Usinally, a good quality triangular engines's ruler contains a sufficient number of graduated scales to fit the requirements of any measurement to be made. The degree of accuracy with which the measurements can be madn will vary directly as the scale that is used. As the seale increases, the accuracy increases.

Finally, the amount of care exercised in plotting the site on the map, and then in measuring its location with respert to certain reference lines will probably have more of an refeet on accuracy than any of the itcms already mentioned.

The plotting of a site onto a map can be done from descriptive data detailing its location, but gencrally it is more accurately performed in the lield where the person doing the plotting can observe how other identifiable. landmarhs nearby are related in dis-
tance and dirertion to the site. A dot ran he madre with a fine-pointed pen or preneil to mark the site on the map, and a fine, half-inch wertical and horizontal lime can be drawn through the dot for casy location later.

Once the site is plotted on the map, one of two mothods may be used to obtain the coordinates of the site. lifther method can be applied to a map drawn to any seale. In the examples to be given here, the sites are plotted on - - 1/2-minute maps.

The sites are first enclosed within a quadrange of lour referener lines drawn through uratieules (crosses) primted on the maps, as shown in Figures 3 and 4. In Figure 3, the four lines drawn throngh thr four graticules to enelose Site I mark the lines of lonvitude at $82^{\circ} \quad 17^{\prime} 30^{\prime \prime} \mathrm{W}$ and $82^{\circ}$ $20^{\prime} 00^{\prime \prime} \mathbb{I \prime}$, and the lines of latitude at $26^{\circ}\left(02^{\prime} 30^{\prime \prime} \mathrm{X}\right.$ and $26^{-\circ} 05^{\prime}\left(00^{\prime \prime} \mathrm{N} . \ln \right.$ Figure 4 the reforence lines enclosing Sit. B are drawn through graticules marhing the lines of longitude at $82^{\circ}$ $20^{\prime}\left(00^{\prime \prime} \mathrm{W}\right.$ and $82^{\circ} 29^{\prime} 30^{\prime \prime} \mathrm{W}$, and lines ol latitudn at $27^{\circ} 00^{\prime \prime} 30^{\prime \prime} \mathrm{N}$ and $27^{\circ}$ $0.3^{\circ} 00^{\prime \prime} \mathrm{N}$.

The lirst method for determining greographical coordinates, the direet reading method, is demonstrated in Figure :3. Between the two latitude reference lines of $22^{\circ} 02^{\prime \prime} 30^{\prime \prime} \mathrm{V}$ and $27^{\circ} 03^{\prime} 00^{\prime \prime}$ N there are ' $2^{\prime} 30^{\prime \prime}$ or a total of $150^{\prime \prime}$. I scalle is selected on the rnginear's ruler so that when the ruler is laid diagonally between the two fatitude reference lines with the scale's dege on Site A, the distance between the two lines corresponds to 150 , or some multiple thereof, graduations on the scalte. In l'igure 3 vach graduation of the scale represents one second of latitude. Aecordingly, Site A lies approximately $53.6^{\prime \prime}$ north of the $27^{\circ}$ $02^{\prime} 30^{\prime \prime} \mathrm{N}$ latitude reforence line. Therefore the latitude of Site A is $27^{\circ}$ $0: 3^{\prime 2} 23.6^{\prime \prime}$ North.


Figure 3. In the application of the direct reading method the geographical coordinates of Site $A$ are read directly from the scale on the engineer's ruler.

The longitude of Site $A$ is determined in a similar manner. Ilowerer, it is advisable that two measurements be made to obtain the longitude of a site: one made with the ruler placed diagonally from upper left to lower right of the two longitude reference lines; the other, from the upper right to the lower left. The reason for making this twofold measurentent is that thre lines
of longitude, though they may appear to be parallel, are not. The lines are actually converging as they approach the north and south poles. Hence, this convergence is detected more readily on maps of areas nearer the polar regions. Topographic maps from the U.S. Geological Survey, in projecting the spherical surface of the carth onto a flat surface, utilize one or another of
two common map projections known as the Lambert Conformal Conic Projection and the Transverse Mercator Projection. Convergence of lines of longitude occurs on maps drawn in either of these projections, but it is more evident on maps using the Lambert Conformal Conic Projection.

Again refer to Figure 3 in which the difference betwren the two reference lines of longitude is equal to $150^{\prime \prime}$. Only one of the two recommended measurements of longitude, from upper right to lower left, is depicted. On some maps there will be no perceptible difference in the two measurments. In which case the longitude is read directly from the scale of the muler. The reading of the scale at Site A in Figure 3 is $81^{\prime \prime}$ west of longitude reference line $82^{\circ} 17^{\prime} 30^{\circ} \mathrm{W}$. So, the longitude of Site A is $82^{\circ} 18^{\circ} 51^{\prime \prime}$ West. If there is a perceptible difference in the two measurements, then the two different scale readings arr added together and their average is taken to be the correct reading. Thus, if the one diagonal measurement of Site A is a scale reading of $79^{\circ \prime} \mathrm{W}$ and the second is a scale reading of $83^{\prime \prime} \mathrm{W}$, then the averaged reading, $81^{\prime \prime} \mathrm{W}$, is taken to be the correct reading.

Some manufacturers of engineering drafting tools offer for sale triangular rulers on which scale graduations are marked in seconds of latitude and longitude. They are specifically dosigned for use on $7-1 / 2-m i n u t e$ and 15 -minute maps. However, the scales of the ruler provide direct readings that are valid only on topographic maps of areas that lie within $49^{\circ}$ North and $49^{\circ}$ South latitudes. The ruler contains three separate scales with which to measure longitude. The scale to be used is dependent on the latitude of the mapped area. Each scale is based on the average amount of convergence of the meridians within
there different bands of latitude, $0^{\circ}$ to $30^{\circ}, 30^{\circ}$ to $41^{\circ}$, and $41^{\circ}$ to $49^{\circ}$. The enginerr's ruler does not suffer such a limitation in its usage.

The second method of obtaining site coordinates from a map is called the ratio method. Again, the site is first located on the map and is enclosed by latitude and longitude reference lines drawn through the printed map graticules. Then two more lines are drawn which intersect at the site. One line passing through the site is drawn parallel to the reference longitude line; the other is drawn parallel to the reference latitude line. Figure 4 is an example of the application of this method.

A scale on the engineor's ruler is selected that will measure with sufficient accuracy the distance between the two latitude and two longitude reference lines. These distances are measured along the two intersecting site limes. In Figure \& the distance between the two latitude reference lines measures 453 divisions on the selected scale; the distance between the two longitude relerence lines measures 406 divisions.

Next, the distance from the site to one reference latitude line and one reference longitude line is measured along the two site lines with the same scale of the ruler. In the example, Site $B$ is 421 scale divisions north of latitude relerence line $27^{\circ} 02^{\prime} 30^{\prime \prime} \mathrm{N}$ and 202 scale divisions west of longitude reference line $82^{\circ} 20^{\prime} 00^{\prime \prime} \mathrm{W}$.

Finally, the latitude and longitude of the site are determined by setting up proportions between ratios of distances measured in scale divisions and in coordinate seconds. The unknown, $\mathrm{X}^{\prime \prime}$, is the number of seconds of latitude or longitude the site lies from the reference lines. Note that the coordinate distance between both sets of reference lines is $2^{\prime} 30^{\prime \prime}$ or $150^{\prime \prime}$.


Figure 4. In the application of the ratio method the geographical coordinutes of Site $B$ are obtained from the solution of a proportion set up between two measurement ratios.

The proportion to detrmine the number of seconds of latitude Site B lies from the reference latitude line is:

$$
\frac{X^{\prime \prime} 1 \mathrm{at}}{150^{\prime \prime}}=\frac{421 D_{\mathrm{iv}}}{4.53 \text { Div. }}
$$

$$
\mathrm{X}^{\prime \prime} \mathrm{Lat} .=\frac{\left(1.50^{\prime \prime}\right)(421)}{(4.53)}
$$

$$
\mathrm{S}^{\prime \prime} \text { Lat. }=139.4^{\prime \prime}
$$

The $\mathrm{X}^{\prime \prime}$ latitude value is added to the relerence latitude.

$$
\begin{aligned}
& \text { Ref. Lat. } 27^{\circ} 02^{\prime} 30.0^{\prime \prime} \mathrm{N} \\
& \mathrm{X}^{\prime \prime} \text { Lat. } \frac{+39.4^{\prime \prime} \mathrm{N}}{27^{\circ}\left(02^{\prime} 169.4^{\prime \prime} \mathrm{N}\right.}
\end{aligned}
$$

Convert 169.4" to minutes and seconds. The latitude of Site $B$ is $27^{\circ}$ $04^{\prime} 49.4^{\prime \prime} \mathrm{N}$.

A similar proportion is set up to determine the longitude of Site B.

$$
\begin{gathered}
\frac{X^{\prime \prime} \text { Long. }}{150^{\prime \prime}}=\frac{202 \text { I)iv. }}{106 \text { I Div. }} \\
X^{\prime \prime} \text { Long. }=\frac{\left(150^{\prime \prime}\right)(202)}{(406)} \\
X^{\prime \prime} \text { Long. }=74.6^{\prime \prime}
\end{gathered}
$$

Add $X "$ longitude to the reference longitude.

Ref. Itong. $82^{\circ} 20^{\prime} 00.0^{\prime \prime} \mathrm{W}$

$$
\mathrm{X}^{\prime \prime} \text { Long. }+\frac{74.6^{\prime \prime}}{82^{\circ} 20^{\prime} 74.6^{\prime \prime} \mathrm{W}}
$$

Convert 74.6" to minutes and seconds, and the longitude of Site $B$ is $82^{\circ} 21^{\prime} 14.6^{\prime \prime} \mathrm{W}$.

## Conclusion

There are other mothods of obtaining site coordinates from topographic maps published by the I.S. Cieological Survey, but they ordinarily involve other coordinate systems in which the grographical coordinates of a station can only be determined by referring to tables in which the coordinates of the two systems are correlated. For evample, on the borders of most topographic maps are printed tick marks which relate lines of latitude and longitude to lines of state survey grids and to lines of the U.S. Army s Unisersal Transverse Mereator (irid. But there hardly seems to ber any peint in resorting to these systems to obtain site coordinates when they ean be determined directly and more simply by one of the methods described previously. Again, the accuracy of dettrmining the geographical coordinates of a site, as required by the FCC, by "ither of these methods will always depend on the accuracy of the map, the accuracy of the scaled ruler, and the accuracy of the measurement.

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## The Volume Unit


#### Abstract

Nearly forty years ago a group of telephone and broadcast engineers in the United States jointly developed and adopted a standard to measure the magnitude of a complex audiofrequency signal in an electrical circuit. The instrument with which such measurements were to be made became known as the Standard Volume Indicator calibrated to measure signal volume (magnitude) in vu (volume units).


T:he volume indicator is an instrument primarily designed to provide a visual indication of the magnitude of non-periodic electrical sigrals such as those which speech and music waves produce in a telephone or radio program circuit. The need for such a measuring device was first recognized during preliminary tests of the public address system set up over telephone circuits that enabled large audiences in several cities of the United States to hear the ceremonies attendant upon the burial of the Unhnown Soldier on Armistice Day 1921. It was observed during some of the tests that distortion due to overloading of an amplifier was more objectionable when heard in a loud speaker than when heard in an ordinary telephone receiver. The speech level at which the amplifiers in the telephone repeaters appeared on the verge of overloading, as evidenced by audible distortion, was determined experimentally. This level, of course, was chosen to be the reference volume against which the volume indicator would measure all other speech signals.

The instrument itself was simply comprised of a potentiometer at its input for adjusting the sensitivity in 2 dB steps, and a triode vacumm tube detector whose output drove a d.c. milliammetcr. The operator of the device adjusted the potentiometer so
that the levels of the measured speech signals produced a maximum movement of the milliammeter needle to the mid-scale point on an average of once every ten seconds. Occasional greater deflections he disregarded. He then read the volume leval from the setting of the potentiometer which was marhed in decibels with respect to the reference volume. (It is perhaps worth noting that the volume indicator provided no control of speech levels in the telephone eircuits; it merely provided a visual indication of the levels.)

During the next several years following its birth, this first instrument spawned a host of similar volume indicators, but there was a lack of uniformity in their design and their use. By the late 1930's the telephone and broadcast industries were acutely aware that this variety of instruments was creating considerable confusion and misunderstanding, especially whenever one group attempted to correlate its measurements with those of another. Standardization was the obvious remedy for this undesirable situation. Though prior attempts at standardization by other associations had failed, during 1938 and 1939 engineers of Bell Telephone Laboratories, Columbia Broadcasting System, and National Broadcasting Company collaborated to develop a new volume
indieator, a new volume reference level, and a new terminology for expressing measurements of volume level. The American Standards Association adopted and published in 1942 the results of this eooperative effort in the form of a standard titled "Volume? Measurements of Electrical Speech and I'rogram Waves."

## Characteristics

In their formulation of the characteristics a gencral purpose volume indicator should have, the engineers gave prime consideration to three uses the instrument would most likely be called upon to serve:
(1) To indicate a suitable level for a speech or program wave so that it is not audibly distorted when transmitted through an amplifier, program eircuit, radio transmitter or the like.
(2) To chech transmission gains and losses in an extended program network by simultaneous measurements at a number of points on particular peaks of transmitted program waves.
(3) To indicate the comparative loudness with which programs will be heard when finally converted to sound.
One instrument to serve all three uses was necessarily a compromise of the ideal instrument for each service.
from the results of extensive and varied tests conducted with trained observers the engineers concluded that there was no appreciable difference between measurements made with rms meters and peakreading meters to indicate speech and program levels at which audible distortion due to overload occurred. Nor did they find any significant advantage in the use of one over the other to determine levels of equal loudness in several types of speech and program material. They did
discover, however, that on a 1,900 mile long program test eircuit that was lined up so that either volume indicator read the same at both ends of the circuit on a 1000 IIz sine wave, there was considerable disagreement between program level readings taken with peak meters at each end of the eircuit. There was only a minor difference in the readings taken with two rms meters. The large variance in the peak meter readings was attributed partly to the cumulative effects of the slight non-linearity in the many vacnum tube amplifiers and loading eooils in the circuit, and partly to the phase changes which alter the wave front and amplitude of the peaks. This marked advantage of the rms instrument conpled with the fact that it could be made of suffieient sensitivity for most purposes without the use of vacuun tubes and their attendant power supplies, an advantage not shared by peak-reading instruments at that time, convinced the engineers to develop the rms type instrument.

In establishing the dynamic and electrical characteristics of the instrument, the engineers sought the opinions of technicians accustomed to reading volume indicators. The conelusions they drew from this survey, as finalized in the American Standard Practice, were that the meter should be slightly less than critically damped so that when a sinusoidal voltage between frequencies of 35 and 10,000 Hz , of such amplitude as to give reference deflection (the point on the meter scale marked $0 \mathrm{vn}, 100$, or both) minder steady-state eonditions is suddenly applied, the meter pointer should reach 99 percent of reference deflection in 0.3 second, $\pm 10$ percent, and should then overswing reference deflection by at least 1.0 percent and not more than 1.5 percent. They felt this slightly underdamped meter move-
ment would cause less eye strain in watching the pointer dance about in response to program waves than a rritically damped or slightly overdamped movement. Moreover, the momentary pause in the pointer's downward mosement permitted more precise observations of maximmm excursions of the pointer to be made.

To further help reduce eyc fatigue the engineers selected a meter seale of butf-colored background sulficiently large to accommodate the number of division markings required to measure the range of volume levels that were likely to be eneounterral. Pigure I is a picture of one such meter seale, the type: A scale commonly used by the telephone industry. The main division markings and the associated numerals above the are denote w's: the less conspicuous scale divisions and numerals printed below the are show percentage utilization of the circuit's volume capacity. On the other standard scale, the type B scale, which the broadcast industry generally prefers, the percentage seale is printed above the are: the vo scale is printed below.

In addition to the meter and its circuitry, the standard wome indicator contains either an attemuator (adjustable loss) or pad (fixed loss). It is important to recognize that when an attenuator is supplied, the adjustable stops marked on its scale are vo's and not decibets of attemuator loss. The su measurement of a signal on such an instrument, therefore, is the algeloraic sum of the vu realing on the meter scale and on the attenuator scale.

Some volume indicators are designed to measure across 1.50 ohm circuits, but most are introded for bridging measurements on 600 ohm circuits, although one version contains an imperlance selector switch which rabales terminated measurements to bee made with the instrument. During


Figure 1. The meter scale on the standard volume indicalor is either the type A scale shou'n here, or the type B scale on which the posilions of the un and percentage seales are reversed.
calibration the instrument is to be connected across the resistive cirenit impedance for which it is designed. I correeth calibrated instrument indicates 0 wo (algebraice sum of meter amd attenuator scales) when a source of sinnsoidal voltagre is adjusted to dewlop one milliwatt in the calibration eirenit impedance.

It should be emphasized here that only sime-wave power ol one milliwatt will indicate 0 va , or reference volume, on the volume indicator. Complea speech or progran waves measured across the stated circuit innerdance whose intensity is such so as to prochuce 0 va readings will have instantamoous peaks of power which are several tincs one milliwatt and an average power which is only a small fraction of' a milliwatt. Reforemer whone, therefore, is properly defined as that level of program which causes a standard wolume indicator, when calibrated and used in the accepted way, to read 0 vo. Noreover, use of the term su as a measure of volume is properly restricted to expressing readings made
with a standard wolume indicator with respect to refermee volume, or 0 va .

## Methods of Reading

Over the years there appears to have developed a difference of opinion as to how the volume indicator is to be read. The engineers who originated thr instrument suggested that the step attenuator, if possible, be so adjusted that the extreme deflections of the instrument needle: just reach a scale reading of zero on the wa scale or 100 on the percentage scale. In their opinion the needle dellactions should be observed for about a one minute period of program material and for about © to 10 seconds of message telephone speech. 'The volume level was read directly from the attenuator scale or, when the attenuator could not be adjusted to give a 0 vu or 100 percent meter scale reading on maximum deflections, algebraically summed with the meter scale reading. The American Standards Practice recommends this same method, but allows the exclusion of one or two occasional deflections of unusual amplitude. Ilowever, the common telephone industry practice is to visually average the peak swings, excluding the highest one or two, over a few seconds interval.

Studies have shown that dilferent observers show different biases in their reading the meter. The range of bias, even among trained observers, is reported to be 3 to 4 w . This observer
hias, when combined with seemingly slight variations in meter reading methods, can result in the transmitted volume in the system being set too high or too low. The transmission system operating under these conditions may exhibit a poor signal-tonoise ratio, or excessive crosistalk, overmodulation of a carrier, or some type of audible distortion.

To some, this reliance on human visual judgement is a marked shortcoming of the standard volune indicazor. Consequently they promote and support the development of other instruments that will give more objective volume measurements. (Others, however, contend that the present instrument adeguately fills the need for which it was intended. Nevertheless, work is groing on now to revise the current standard practice and broaden its application. It is likely that the revision will definte the characteristics of other volume indicators in addition to the current standard to permit the application of current technology to this type instrument. But it is unlikely the revised practice, in defining several volume indicators as standard, will werlook the necessity for a simple method of correfating measurements made with dilferent instruments. Oherwise, the telephone and broadcast industrits will be confronting again the situation that originally prompted them to develop a standard volume indicator.

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